

APPLICATION OF MODELS IN GEOHYDROLOGICAL INVESTIGATIONS

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ABSTRACT

The application of models in geohydrological investigations is closely related to the use of quantitative methods in groundwater hydrology. To help determine which type of model should be chosen an analysis of the following aspects is required: nature of the problems, structure of the hydrological system, types of models and general conditions. The large number of factors involved in this analysis, and the small number of results of alternative methods of investigations, preclude the formulation of a generally applicable rule of selecting a model.

INTRODUCTION

The first calculations of groundwater flow, based on Darcy's law of 1856, were published by Dupuit in 1863. Since then many thousands of publications have followed including early contributions by Thiem (1870) in Germany, Lembke (1866) and Zhukovskii (1888) in Russia, Forchheimer (1886) in Austria, Badon Ghijben (1899) in The Netherlands, Slichter (1898) and King (1899) in the U.S.A., and Boussinesq (1904) in France. The most fundamental of these publications were perhaps those of Forchheimer, Zhukovskii, Slichter and Boussinesq. Reviews of the early literature, supplemented by later contributions

may be found in more recent publications by Muskat (1937), Polubarinova-Kochina (1952), Aravin and Numerov (1953), Harr (1962), Bear et al. (1968) and DeWiest (1969).

Model investigations applied to geohydrologic problems were introduced in the beginning of this century (Pennink, 1905), but in most cases analytical methods of solution remained in more general use. Around 1845 Kirchoff hit upon the idea to study potential fields with the help of conducting sheet material. In 1899 Slichter pointed out the analogy between the mathematical expression of flow of water in soil (Darcy) and flow of electricity in conducting media (Ohm). Pavlovsky, in 1918, was the first to use this analogy by means of a conducting sheet of tin. Around 1930 Vreedenburgh and Muskat used an electrolyte as a conductive medium. This was already done before them by Gugin (1862) for a two-dimensional case and Adams (1875) for a three-dimensional case, but for other purposes.

The viscous flow or crevice model of Hele-Shaw (1897) was introduced by Dächler (1936) to study groundwater flow. This model is an excellent tool for studying a steady or moving interface between fresh and salt water, for instance sea water intrusion (Santing, 1951). By using a wide crevice and filling it with sand (d'Andrimont, 1905) or with glass beads unsaturated flow may be simulated (Miller and Miller, 1956, and Kraijenhoff van de Leur, 1962). A horizontal Hele-Shaw model with storage was applied for the first time in

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1956 by Kruyssen and Bear to study groundwater flow. The principle is similar to that of the Hydraulic Analog constructed by Moore in 1936. The principle is similar to that of the Hydraulic Analog constructed by Moore in 1936.

Electrical resistance networks for the purpose of studying groundwater flow were probably first applied by Luthin (1953). However in other fields such networks have been used much earlier (Taylor and Sherman, 1928). Beuken (1936) is known to have made the first resistance capacitance (RC) network in 1932, with which transient heat flow phenomena were investigated. This type of network is known as the Beuken model.

As opposed to the analogues mentioned earlier, the distribution of a function in a resistance network is not continuous along a space variable, but discretized in the network's nodes. This network of resistances can be compared with a mathematical network, to which the finite differences method is applicable (Depackh, 1947 and Liebmann, 1950). Errors caused by discretization of nodes may be determined by the Taylor series (Verspuy, 1970; Volinskii and Buchman, 1965; Stallman, 1963; Karplus, 1958; Landau, 1957 and Huard de la Marre, 1953).

The appearance of the computer enabled a digital solution of complicated hydrological problems. At the outset, the computer was reluctantly used, but after 1960 the number of its applications grew considerably, while electrical networks were less employed. Around 1965 the interest in using electrical networks grew again. Both methods have their advantages, drawbacks and limitations and as these do not lie parallel, a combination of the two is rather obvious. The hybrid method, based on this combination, (Ichikawa, 1970; Parker, 1969; Renard, 1969; Vemuri and Dracup, 1967) is not often applied yet but offers the widest scope.

This historical survey of the various methods applied to solve problems, is not complete. It would also not be right to draw the conclusion that less preferential consideration should be given to an older than to a more recent method. Application of all methods leads to a flexibility of mind, which, in turn, often means a deeper insight into the hydrological mechanism.

A selection of methods is, of course, needed. Insight into decisive aspects is the basis for a justified

choice of method. These aspects are:

- nature of the problems
- structure of the hydrological system
- types of models
- general conditions

In this article various types of models will be reviewed and discussed. Being well aware of their limitations, the authors hope that the bibliography at the end will serve those desirous to pursue this subject in greater detail.

NATURE OF THE PROBLEMS

A distinction is made here between *economic* and *hydrological* problems. In the group *economic* problems the question often is to determine the efficiency of a hydrological system, and stress must be laid here on deciding the economic optimum. This optimum depends a.o. on the flexibility and sensitivity to calamities of the system, while socio-economic factors must not be forgotten. It is assumed here that alternative hydrological systems can be fully translated in one or the other model, which makes it possible to determine the output from the input, either in the form of stochastic quantities or not. No further discussion of economic problems is included in this paper.

The primary object of the group of *hydrological* problems is the determination of hydrological quantities such as piezometric head, drawdown, hydraulic gradient, seepage, discharge, recharge, leakage factor, transmissivity, storage, time lag and damping. In this enumeration is tacitly assumed that the formulation of the hydrological system is known and can be translated into a model, in the widest sense of the term. It often happens that the model itself is the subject of investigation. Following Kozdoba (1969) and Hackeschmidt (1965) this kind of investigation is called *inductive* investigation as opposed to *deductive* investigation, where the model may be regarded as known.

Deductive investigation can be divided in four sub-groups of problems: *direct*, *indirect*, *reverse* and *inverse*. A short description of these concepts follows here.

Direct problems

This group comprises all problems related to the determination of the distribution of the piezometric

head or the drawdown in a hydrological system, of which the dimensions, boundary and initial conditions and the parameters (c.q. transmissivity, storage coefficient, leakage factor etc.) are known. A typical example of this is the determination of drawdown patterns as a consequence of groundwater abstraction. Other examples are a.o. determination of flownets under and/or through structures, or seepage from or towards reservoirs, canals, rivers, ditches and wells.

Indirect problems

This group includes the problems related to the determination of a boundary, on which the conditions are known, and that is part of an otherwise completely formulated hydrological system. Classic examples of this are the determinations of a phreatic and a seepage surface. The determination of the propagation of an influence boundary of e.g. a multiple well system is also regarded as an indirect problem. A typical problem is also that of the interface of fresh and salt water in coastal aquifers. As indirect problem may further be mentioned the design of gravity drainage systems.

Reverse problems

Reverse problems are those related to the determination of boundary conditions in a hydrological system, where all other quantities are known. The determination in a watershed of the distribution of the precipitation excess or evaporation (negative excess) is a well-known example. To this group belongs also the problem of water-levels determination in irrigation systems to obtain the percolation wanted. Problems of artificial recharge are also seen as belonging to this group if the matter at issue is the determination of the pressures needed in the wells to attain the capacity required.

Inverse problems

Inverse problems can be defined as problems related to the determination of parameters, e.g. transmissivity and storage coefficient. This determination is done with data, derived from a known pattern of groundwater flow. A simple example of this is the determination of the transmissivity from the parabolic distribution of the piezometric head of a steady

two-dimensional flow towards two parallel canals caused by a known precipitation. All kinds of determination of aquifer characteristics by pumping tests and determinations of permeability by piezometer methods or auger holes can be regarded as inverse problems.

In conclusion the multiple regression method may be mentioned. This is the method to determine parameters by way of a mathematical model. For this purpose the "concomitant variate" can be applied too instead of the "regression method". Also the maximum likelihood criterion can be used instead of the least squares.

STRUCTURE OF THE HYDROLOGICAL SYSTEM

The complexity of a hydrological problem is for a very large part determined by the structure of the hydrological system. The factors, characteristic for a system will be gone into briefly. As the groundwater flows are regarded as belonging to the group of potential flows, the following factors can be distinguished: *medium, dimensions, parameters, boundary and time conditions.*

Medium

By medium is understood here the porous medium. The influence of this medium on the flow of water is expressed in the coefficient of permeability. Depending on the way in which these porous media have been formed geologically, the permeability varies strongly in space. The scale, to which the variations relate, depends on the extent of the area in which the flow is the subject of investigation.

In drainage problems this is a matter of fractions of one meter to some ten meters and in seepage problems from ten to hundreds of meters. The dimensions of the area in basin studies can be as large as many kilometers. The concept *heterogeneous* cannot be dissociated here from the dimensions of a hydrological system. In the case of large dimensions heterogeneous can refer to the presence of strata with a different permeability, though each stratum may be homogeneous. Besides the variation in permeability from spot to spot, it can also be a case of direction dependence. If so, it is an *anisotropic* medium. The coefficient of permeability is in fact a tensor and not

a scalar quantity. The distribution of the anisotropy should also be seen in relation to the dimensions of the hydrological system.

Dimensions

In this context dimensions are the *space-variables*. The complexity of a problem grows with the number of dimensions. As the aim has always been to find a method of solution with the lowest cost possible, it will be necessary to determine whether the number of dimensions can be justifiably reduced. The picture of the flow will then deviate a little from the flow occurring in situ. The consequences in regard to the accuracy of the solution will not be precisely known but can often be estimated. The reliability of the estimation depends strongly upon the ability of the hydrologist. In some cases reduction in the number of dimensions is even necessary, because the problem could otherwise not be solved. Besides reasons of economy and necessity, simplification for the sake of clearness can be a reason for reduction of the number of dimensions.

In the following considerations the number of dimensions will be defined as the minimum number of dimensions necessary, with which water movement in the soil can be formulated. This number varies between three and nought. By a number of nought dimensions the groundwater flow is formulated in quantities, in which the space-variables do not occur. The quantities can be defined in various ways, e.g. as a mean or as a quantity representative for a certain area. In most cases such formulations are deduced from analytic solutions of schematized one- or two-dimensional groundwater flows. Finally, it may be observed that simplifications (c.q. reductions in the number of dimensions) can also be obtained by dividing the area into sub-areas and to apply a reduction in each sub-area. In this way a two-dimensional flow through a semi-confined aquifer can for instance be reduced to two one-dimensional flows.

Parameters

In the scope of this article parameters are the *multiplicative factors* in the differential equation of the groundwater flows. All physical properties of the porous medium in relation to the flowing of water, such as permeability, storativity, diffusivity etc. are

expressed by these parameters. The coefficients of the physical properties in the parameters often occur in combination with a characteristic length in the area. Examples of this are transmissivity, layer resistance, leakage factor, time lag etc. In most cases the value of the parameter can be regarded as constant. In zones, for instance, where the saturated flow changes into a non-saturated one, great differences in value of the parameters may occur. In aquifers, which are not constant as to thickness, the value of the transmissivity varies too. As stated in the explanation on the dimensions the complexity of a problem grows with the number of variable parameters.

Boundary conditions

Boundary conditions can be grouped in *linear* and *non-linear* conditions. The *linear* ones can be subdivided again in three groups: the Dirichlet, Neumann and Fourier type. In the Dirichlet type the value of the function is prescribed on the boundary, in the Neumann type the value of the gradient and in the Fourier type a linear connection between the values of the function and the gradient. When a canal is a boundary and its water level does not vary, it is a typical example of a Dirichlet condition. In pumping tests with a constant rate of flow it is a Neumann condition. The Fourier condition is applied in a semi-confined flow if the rate of leakage is proportional to the drawdown. The same situation arises if in the example of the Dirichlet condition the water level in the canal is assumed to be proportional to the discharge from the canal. A *non-linear* relation occurs for example when taking into account the influence of the evaporation upon the phreatic surface. It can be said here too, that the complexity grows with the number of non-linear boundary conditions and that in solving the problem a reduction of this number may be necessary.

Time conditions

Time conditions refer to *steady* and *non-steady* situations. Two kinds of non-steady conditions are distinguished here, namely the *initial* and the *periodic* conditions. For the *initial* conditions there is always a point of time to be given on which an alteration of an existing condition is made. By existing condition is mostly understood a steady state situation. The point

of time when a new condition takes effect is usually regarded as initial point of time and expressed by "t = 0". Initial conditions, for example are applied in formulating the drawdowns during a pumping test. Alterations in the level of a ditch or in the weather conditions (rain) are considered initial conditions. The *periodic* condition on the other hand does not have an initial point of time, but here there is a condition, that the distribution in time of the value of a function repeats itself in a fixed, regular and prescribed pattern. The repetition time is called the period for short. Though we hardly ever come across exact periodic conditions in nature, they are used as approximations to formulate groundwater flows, which are influenced by e.g. the tide. Alterations as a consequence of seasons, succeeding each other, can be approximated with the help of periodic conditions. Fourier series can be used then.

The problems to be solved are often considerably simpler when the function values do not vary with time. Though this situation never arises in nature, the steady state condition can be accepted as a right approximation of an average of a more or less periodic condition. Steady state solutions are also applied to obtain an approximated solution of non-steady flow problems in the method of successive changes of steady state conditions. If a non-steady flow can be justifiably reduced to a steady flow, preference will be given to solving the steady flow.

TYPES OF MODELS

Every calculation method conceives a real hydrological mechanism in a form suitable for quantitative considerations. This is, in fact, what is meant by the term "model", which term will be used from now on. To distinguish between the various types of models a sub-division into two groups can be made, namely the *abstract* (conceptual) and the *concrete* (physical) models. A brief explanation of these models follows here.

Abstract (conceptual) models

All translations or formulations into mathematical symbols of hydrological mechanisms – i.e. mathematical models – are considered to be abstract. The

group of abstract models can be sub-divided into *Continuous* and *discrete* models. Continuous models are based on differential equations, to which continuously distributing function values are applicable. Solutions can be had either through an *analytical method* (e.g. direct integration, integral transforms, superposition, Green's functions, conformal mapping and variational calculus) or by an *analog computer*. The analytical method leads to abstract solutions (formulae) and the analog one to graphic solutions (curves), which can only be obtained after all quantities have been expressed in numerical values. *Discrete* models proceed from finite difference equations, to which only discrete (numerical) function values are applicable. Solutions can be obtained either through a *graphic method* (e.g. small squares) or the *relaxation method* or *digital computers*. All three methods give particular solutions: in the graphic method in the form of a sketch and in the other methods in a table which can be translated into one or more curves.

As opposed to digital computers, analog ones can only be fed with problems having one independent variable. This means that partial differential equations must be reduced to ordinary differential equations, either by separating the variables or by substitution (Boltzmann).

Concrete (physical) models

Simulation, often on a smaller scale, of a real hydrological mechanism or use of other physical phenomena analogous to laws of the hydrological mechanism are considered to be concrete models. The first are indicated by homologous models or *homologues* or so-called *sand boxes* and the latter by analogous models or *analogues*. The analogues can be subdivided again into *continuous* and *discrete* analogues. The magnetic, membrane, heat, Hele-Shaw, Christiansen filter, ion-motion, electrolytic and electrical conducting sheets, gelatin or plastic analogues belong the continuous analogues. The hydraulic and resistance networks are typically discrete analogues. Both types often occur in combination as is the case in a horizontal Hele-Shaw model with discrete storage and in conducting sheets with a finite number of capacitors and/or resistors.

Not all models are suited for non-steady flow problems or for the study of flows with different

densities. This is why a.o. the magnetic and membrane analogues are hardly ever applied. Due to P a s c h k i s (1959) who used a liquid in which the conduction is performed through free electrons – as occurs in metals – the electrolytic tank may also be used for solving non-steady flow problems. For the study of salt-fresh water problems, the Hele-Shaw model is very well suited. In 1969 Dagan introduced an approximation method with which these problems can also be solved by using an electrical network.

Electrical analogues are preferred mainly because a great diversity of boundary and time conditions can be simulated this way. Moreover, electrical measuring instruments have a wider scope than other kinds of measuring instruments. The electrical analogues applied most often consist of electrical networks, composed of resistors, combined or not with capacitors, and/or semi-conductors. The structure of such a network is similar to that of a mathematical model used in the digital solving method. Errors caused by discretion can be calculated in the same way as is done in mathematical networks (Taylor series). This similarity is also the basis of the possibility, in principle, of coupling an electrical network to a digital computer. This combination of network and computer is called a *hybrid computer*, a synthesis of abstract and concrete networks.

GENERAL CONDITIONS

The solution of a problem is not decided by the aspects discussed in the previous chapters alone. Certain general conditions have to be taken into account as well. These conditions and the aim of the investigations are closely related. Though an analysis of aims will deepen the insight in the choice of the solving method, in this article only a number of factors are mentioned. These factors can be summarized in four groups: *quality, presentation, use and organization*.

It may be observed that division into groups does not imply that the factors from the different groups are not interrelated. Neither is pretended that this survey is complete.

Quality

To this group belong:

- a. accuracy
- b. reliability
- c. validity

The *accuracy* of the result of a calculation or measurement is decided on the one hand by the inaccuracy of the active part of the model (input) and on the other by the inaccuracy with which the parameters, the passive part of the model, have been determined. It should be tried, wherever possible, to find out what influence any inaccuracy may have on the accuracy of the result. This gives an impression on the “weight” of an inaccuracy and of the consequences a certain accuracy has in regard to the result. These consequences may sometimes lead to a simplification of the input and/or the determination of the model’s parameters, and therefore to a saving of costs.

The *reliability* aspect is determined exclusively by quantities of a stochastic character. The confidence interval can be determined by statistical methods.

By *validity* is understood the degree of similarity between the assumed and the real mechanism, which makes validity a measure for identity. Identity can only be decided through verification. Validity is also decided by the range in which the variables vary. When translating non-linear mechanisms into linear ones, an acceptable degree of similarity can be found by accepting a small range for the variables. To get an impression on the behaviour of a hydrological system a model of limited validity often suffices.

Presentation

By this term is understood the form in which the results or output are presented. The following three forms are distinguished:

- a. formula
- b. curve
- c. table

The demand that the result should be given in the form of a *formula* will not often be met. In translations of hydrological systems based on similarity in behaviour and not on similarity in formulation of the mechanisms (c.q. differential equation), the result can hardly ever be given in a formula.

An empirical formula may be found through dimensionless numbers by including in the considerations either differential equations or a dimensional-

analysis. Exponents will have to be determined by regression methods. The demand to present the result in the form of a formula can be met completely in the case of an analytical solution. Though this solving method has been developed quite strongly, particularly in the USSR, its application is limited. The main advantage of a formula, not consisting of a series, is the possibility of a quick recognition of the tendencies of the various factors, providing a better insight into the behaviour of a system.

The presentation of the results of calculations or measurements in *curves* is an essential aid to judge the results of an investigation and to draw conclusions. This visual presentation gives in one glance an insight into the various aspects such as mean value, maximum and minimum, gradients, periodicities, damping, delay and irregularities. Output in the form of a curve is obtained immediately with analog computers and (electrical) analogues.

When a digital computer is used an interface after the output is needed; either a scope with a DA-converter or a plotter or grid screen can be connected with the interface. As not every institute, owning a computer, has such apparatus the condition that presentation should be done in the form of a curve may mean unacceptable delays.

Presentation in the form of a *table* is usually obtained when using a digital computer. If other types of models are applied a printer should be on line after the output. A table is well suited to record quantitative information, e.g. mean, maximum and minimum values, correlation coefficients, frequencies etc. A table is less suited for the display of qualitative information than a curve. The accuracy of the information is practically unlimited in a table as opposed to a curve, where the smallest error is already about 0,5%. In order of magnitude this error corresponds with the smallest error occurring in analogues. The accuracy desired may have a considerable influence on the choice of model and the presentation of the results.

Use

This term comprises the group of demands which a model should meet in regard to flexibility and frequency of application. The term "use" can be sub-divided with regard to:

- A. flexibility:
 - a. universal (comprehensive)
 - b. specific
- B. frequency:
 - a. once
 - b. number of repetitions
 - c. continuous (demonstration or production)

In this connection the concept "*universal*" or comprehensive is hard to define. If a model in which the configuration of the hydrological system is fixed and in which both the values of boundary and time conditions and of the parameters are the only variables, it is not an universal but a *specific* model. In case not only the above mentioned values but the configuration and the meaning of the boundary condition can be varied as well, the model may be interpreted as "comprehensive" or "universal".

In an universal simulation model the number of sources of errors is generally larger than in a specific model because of its flexibility. Besides, gauging of components is not so easily done. The many gliding or interruptive connections may influence the stability of the model unfavourably. In simulations of three-dimensional flows by universal electrical analogues the many long connection wires often cause failures. Generally, the advantages of an universal application do not outweigh the drawbacks. These drawbacks do not appear when digital computers are used. Then the computer itself is the universal model and its program corresponds to the rules valid for other types of models.

With the help of simulation languages (e.g. Simscript) problem reading by computer is considerably simplified. The number of possibilities is often a limiting factor, particularly with regard to the number of nodes. The difference between the algorithmic character of a computer and the physical character of an analogue remains a gap with respect to simulation of physical phenomena. This gap can be bridged only by using hybrid computers, where the analogue simulates the physical phenomena and the computer takes care of the input and the processing of the output. Development of universal solving methods is expected to go in the direction of hybrid computers.

At present the *frequency* with which one and the same type of problem is solved does play an important part when choosing an universal (digital computer) or a specific model (analogue). For smaller

frequencies and for the solution of very simple problems analogues will be preferred. Large size problems (large number of nodes) can as yet better be solved with the use of analogues. A frequent or continuous application of a solving method with a small or not too large a number of nodes will preferably be done by digital computers. Analogues are extremely suited for demonstration purposes. In all other cases, not mentioned above, the choice of method is in a much larger degree influenced by other factors.

Organization

Organization comprises a diversity of factors, among which are:

- a. knowledge
- b. availability of equipment
- c. possibility of realization
- d. time of realization
- e. training of personnel
- f. cost
- g. length of life
- h. exploitation

Previously gained experience is, naturally, very important when choosing a type of model for solving hydrological problems. So is the equipment purchased. For, once a group of experts has been formed which disposes of good equipment it can hardly be expected that a hydrological problem is going to be solved in another way than this group usually does. The difficulties of retraining personnel, gaining of knowledge in another field and purchasing of new equipment usually discourage changing established methods. Therefore, a narrow development offers a small number of possibilities of applying alternative solving methods. Besides, the choice of a solving method is determined by the period of time, within which a solution of the hydrological problem should be found. This period includes time of development, programming, execution and processing of results. An important aspect is here whether these time-consuming activities are carried out by one or by various authorities. This often implies that personnel of the one authority has to specifically trained by the other one. Then, choice of the solving method is not only determined by the time of realization but by the specialization present in the other organization as well.

Finally, the cost aspect must not be disregarded. Realization and, possibly, training cost, length of life of a model, particularly if it is frequently used, are all essential factors with regard to the exploitation of a model. Unfortunately, little information is available on the cost aspects.

FINAL DISCUSSION

The aspects determining the choice of the solving method are shown in table 1. The meaning of each aspect has been explained in the previous chapters of this article. In the scheme two main groups are distinguished. The first refers to the *formulation* of the problem and the second to the *selection* of the method.

In this grouping is proceeded from the idea that the problem posed is not solved if the solving method indicated is exclusively based on theoretical possibilities. In reality the client makes certain conditions with respect to the solving method. These conditions are, naturally, closely related to the aim of the solution. It is in the nature of things that other conditions are made if the aim is purely scientific. It happens more often than not that a solution is found which meets only a number of demands. Compromise proposals have to be put to the client. It is not exceptional that is agreed upon an alternative solving method. These negotiations will be more successful the larger the number of solving methods offered is.

On the other hand, efficiency asks for a limitation in number of methods. An optimum number of solving methods must be selected, depending on the market and the competition. Unfortunately, essential data for doing this are lacking just yet, making it impossible to formulate unequivocal rules. Some directives, not based on the general conditions, may be given.

In *inductive* problems the model is unknown and the number of unknown quantities is larger than the number of equations available. The missing equations will have to be traced by the theory of systems, where the information theory is very important. Both digital and analog computers are used to this end. For instance in flood routing and runoff calculations the results have been quite satisfactory.

In the group of *deductive* problems the formulation of the model is assumed as known. In reality, an

TABLE 1
Formulation of the problem and Selection of the method in geohydrologic investigations.

SOLUTION =			
FORMULATE THE PROBLEM		+	SELECT THE SOLVING METHOD
Problem	Hydrological System	Model	General conditions
Deductive direct indirect reverse inverse Inductive	Medium homogeneous heterogeneous isotropic anisotropic Parameters constant variable Dimensions three two one nought Boundary conditions Dirichlet Neumann Fourier Non-linear Time conditions steady state non-steady state initial periodic	Abstract (conceptual) continuous analytical analog computer discrete graphical relaxation digital computer Concrete (physical) continuous sand boxes heat analogue membrane analogue viscous analogue ion-motion analogue electrical analogue sheet electrolyt gelatine plastics discrete hydraulic analogue stor. visc. analogue electr. netw. analogue Hybrid analog comp. + digital comp. electr. netw. + digital comp.	Quality accuracy reliability validity Presentation formula curve table Use flexibility universal specific frequency once number of repetitions continuous Organization knowledge equipment possibility of realization time of realization training cost length of life exploitation

inductive problem often precedes the solution of deductive problems as the model of the real flow is not known. In some cases even the boundary and time conditions are unknown. In such situations a combination of various kinds of problems exists. More insight is obtained when the various problems are separated.

In *inverse* problems the magnitude of the parameters must be determined. This is often done by drilling operations combined with pumping tests. These pumping tests introduce artificial disturbances, the effects of which can be formulated mathematical-

ly. The parameters may then be calculated through various techniques. A different method is flow simulation by a model with variable parameters. The model's response is compared with the real response. The parameters in the model are changed in such a way that the best possible fitting is reached between the model's response and the reality. This makes the availability of an universal model necessary. When an electrical (RC) network is used the fitting is based on visual interpretation, which is a *subjective* criterion. The criterion becomes *objective* when digital computers and optimization techniques are used. The

number of nodes is limited if the permeability in a node does not have the same value for the various directions. Besides, the storage in nodes (e.g. surrounding the phreatic surface) must be added to the number of unknown parameters related to the permeability. In the objective method the number of nodes will be some tens at most. This number is in general small, so for a large number of nodes the criterion will become subjective again. Depending on the complexity of the flow the number of nodes in these cases amounts to a few hundred for a digital computer and to a few thousand for an electrical network. Moreover, an electrical network gives the information wanted sooner and its manipulation is simpler than that of a digital computer. The determination of the magnitude of the parameters becomes considerably more complicated if the parameters are a function of e.g. the potential. In these cases the digital computer is preferred. For both techniques may be observed that the real nature of the flow, is not always fully known. The results should always be judged against the background of this limited validity, and tested for its physical significance.

In *reverse* problems time and boundary conditions must be determined. For steady state or periodic conditions the boundary conditions can be determined on the understanding that they do not belong to the group of non-linear conditions. By regarding the values of periodic conditions as averages the problem can be reduced to a steady state one. In reverse problems the model and the functions within the boundaries may be assumed as known.

In a simple case the boundary conditions for instance can be determined with the help of equipotential lines. In more complicated cases the boundary conditions can be calculated by extrapolation or by momentum and continuity equations. The trial-and-error method is less systematic. In solving reverse problems special preference is neither given to abstract nor to concrete models.

A model with flexible boundaries is needed to solve *indirect* problems. Both for digital computers and electrical networks the form of the boundary is influenced by the form of the meshes of the network. This influence is small for irregular boundaries as opposed to boundaries with smoothly curved planes. In simple models electrolytic tanks and sheet models, where the electrodes consist of mercury, meet the

flexibility demand best.

The description of a flow in a formula (analytical model) is the ideal solution. Hooghoudt's formula is the simplest example of this. In this formula the distance between the drains can be varied continuously, so can the depth of the impermeable stratum and the diameter of the drain. There is no preference for any special model also for indirect problems. A choice will have to be made depending on type of flow and form of boundaries.

As opposed to the foregoing a different method of explanation will be used for *direct* problems. Some possibilities and limitations of each type of model will be mentioned to make it easier to compare the merits of the various types of models.

a. *Analytical* solutions exist for relatively simple three-dimensional steady flow through a homogeneous and isotropic medium. For more complicated problems the solutions are limited to two-dimensional flow through a more or less isotropic layer or through non-isotropic layers. For non-steady flow acceptable approximation solutions are known, which are mostly found after reducing the number of dimensions. Anisotropy is not an unsurmountable difficulty on the condition that the flow is two-dimensional and the medium homogeneous. In some cases solutions have been found, in which the parameters are not constant. Particularly in Russian literature many analytical solutions are described.

b. *Analog computers* are successfully applied in simulating the interactions of the various hydrological systems with non-linear relations and with relations depending on time. The influence of the space-variable is translated by dividing the hydrological system in sub-systems. The output is an electrical signal that can be recorded either analogously or digitally. The accuracy is less than that of a digital computer but considerably greater than that of analogues. The problem to be solved must be formulated in a mathematical form. These computers are used particularly in the USA, but also in Europe.

c. *Graphical* solving methods can all be reduced to the method of the small squares, which method is exclusively applicable to two-dimensional steady flows through homogeneous strata. Though the result is obtained by trial-and-error an inaccuracy of less

than 1% cannot be excluded. For this method mathematical knowledge is not needed. Its frequent application contributes to the imagination indispensable for the solving and adjustment of hydrological problems.

d. *Relaxation* methods are based on interpolation. In the nodes of a grid the value of a function is calculated with the aid of differential equations by trial-and-error. Though the relaxation method can be applied to three-dimensional flows, this method is mainly used for solving two-dimensional steady state problems. The heterogeneous character and the anisotropy can be taken into account. Some mathematical knowledge is required for this method. The order of magnitude of the inaccuracy can be determined by a Taylor series depending on size and form of the grid's meshes and the distribution of the function. The inaccuracy can be reduced to less than 1‰. A picture of the flow is obtained by drawing the equivalent lines.

e. *Digital computers* can be regarded as programmed executors of the algorithmic method. Though the use of simulation languages simplifies the operation of digital computers, specific training remains indispensable. The possibilities of these computers seem almost unlimited, but the algorithmic character restricts the number of nodes. A directive limit is a number of a few thousand nodes. The complexity of the problem to be solved influences this number only slightly. In most cases this limit does not meet objections. All combinations of the aspects, as mentioned in the second column of the scheme (table 1), are applicable. The advantage a digital computer has, is not only that it can solve combination problems as meant above, but that it can carry out all other kinds of arithmetical operations too. The possibilities of a digital computer offer important advantages, particularly in calculating frequencies or optimization procedures and in determining the efficiency of a hydrological system for purposes of management. The inaccuracy is not so much determined by computer as by choice of magnitude of increments applied as a consequence of discretion of space and time variables.

The lack of sufficient contact between hydrologist and the physical phenomenon simulated by computer is often seen as a drawback. For instance visual

judgement of the influence of a continuous variation of a parameter cannot be done. For each value of the parameter the computer carries out a calculation and only at the end of a run a new value of a parameter can be entered. This limitation may be an objection when examining the behaviour of a physical system. This is often a reason to prefer an analogue in spite of the many advantages of the digital computer.

f. *Sand boxes* are in fact a smaller scale (homologous) reproduction of the system in reality. They are the oldest known models. The difficulty of these models is mainly that the properties of the porous medium can hardly ever be reproduced. Besides, capillary action of the medium may have a disturbing influence on the phenomenon to be examined. This influence may largely be neutralized by using liquids with a small surface tension. Fixation of the matrix of the porous medium may be a way to escape the low reproduction ability. At all times it must be carefully avoided to entrap air in the porous medium. A liquid free of air must be always used. The measurement and simulation of phenomena is also difficult. By using glass beads instead of sand some of the difficulties of a porous medium may be eliminated. To get a picture of the flow pattern in the inside of the model the porous medium will have to be transparent. This can be realized by using the *Christiansen filter*. Its principle is based on the equalization of the refraction index of the liquid and solid. So crushed glass or glass beads are applied as porous medium and oil or aqueous solutions as liquid. The poor flexibility of the construction remains an objection. Sand boxes are seldom used nowadays because the advantages of these models are small.

g. *Analogues* of various types occur (table 1). As the range of applications of heat, membrane, ion-motion, gelatine, plastic and hydraulic analogues is only small, we will not go into it. As mentioned earlier, the *viscous analogues* are eminently suited to study fresh-salt water problems and flows with a phreatic surface. Only two-dimensional flows can be simulated. Strata with a different permeability are obtained by choosing different thicknesses of the crevice. They are also suitable for studying non-steady flows. It is the only analogue in which the interface and phreatic level adjust themselves automatically. The drawbacks of the model are the poor

flexibility and the difficult accessibility to carry out measurements. In manufacturing the model the openings for the piezometers must already be made. The model is very illustrative and is therefore often used for demonstration. The model is used both in vertical and horizontal position. In the latter position the storativity can be simulated by a network of standing pipes, connected to the upper plate. These pipes also act as piezometers.

Electrical analogues are more and more applied for hydrological investigations. Two-dimensional flows are mostly studied with the aid of *sheet* analogues. A small layer of carbon is the conducting medium. Electrical connections are made with the use of silver paint. Another possibility is the application of mercury contacts which can be moved over the carbon layer to simulate flexible boundaries. The Western Europe sheets are heterogeneous and anisotropic up to 10 to 15%; the Eastern Europe sheets can be practically homogeneous and isotropic. The latter are difficult to obtain. Sheets can be made heterogeneous artificially by perforating them. A regular distribution of the perforations can reduce the permeability four times at most. By piling the sheets the permeability can, in principle, be increased without a limit. In practice however the limit is about 10 to 20 layers. A change in permeability can also be obtained by preparing the sheets with a solution, but if this is done the reproduction ability sometimes leaves much to be desired. Boundaries, on which the potential is not constant, can be simulated with resistance paint. By exchanging stream and potential boundaries the flow lines can be easily determined. Simulation of storativity is done by pasting the sheet with an isolating layer (dielectricum) on a metal plate. This makes it possible to solve non-steady problems too. The inaccuracy can be reduced to a few percents, depending on the kind of output, in spite of the fact that the sheet is heterogeneous and anisotropic by nature. These analogues are very simple to make and though they have their limitations they are well suited to solve two-dimensional flows with irregular boundary conditions. The active part (power supply, function generator, etc.) and the measuring apparatuses (voltage meter, oscilloscope, recorder etc.) are the most expensive items.

Three-dimensional flows can be simulated with the *electrolytic tank* because the conducting medium is a

liquid. If tap water with a saline solution is used an alternative current must be applied to avoid a.o. polarization. This makes it impossible to study non-steady flows, having a D(irect) C(urrent) component. This objection may be met by applying the liquid recommended by Paschkis (Tetrabutyl Ammonium Picrate mixed with Dowtherm of Dow Chemical). As a consequence of contact potentials the model voltages amount to more than 100 Volt if an inaccuracy of less than 1% is required. Zones with different permeability (different salt concentration or different depth) can be made with partitions fitted with rivets or pins. Boundary conditions are simulated by metal electrodes. If the boundary is an equipotential surface the electrodes can be replaced by a metal plate. Building an electrolytic model is relatively time consuming. The model is eminently suited for studying the three-dimensional flows through a homogeneous and isotropic medium, while the form of the boundaries may be very irregular. The inaccuracy depends strongly on the cleanness of the electrodes. With careful execution the inaccuracy can be less than 1%.

In *electrical networks* the conducting medium is composed of a network of resistors. By adding capacitors non-steady phenomena can also be studied. If non-variable resistors and capacitors are used, networks with 10000 or more nodes can be made with relatively simple devices. Naturally, both the heterogeneity and the anisotropy can be taken into account in a rather simple way. By using semi-conductors a certain degree of non-linearity can be simulated. Also in this model the active part is the most expensive. For the study of three-dimensional non-steady groundwater flow problems, where anisotropy and heterogeneity must be taken into account, the electrical networks have the widest scope and are even the only possibility if the number of nodes is more than about 5000. The magnitude of the inaccuracy in this network can be determined also with the use of a Taylor series. It may be observed that a network can be combined with a conducting sheet or an electrolytic tank. To calculate the inaccuracy then becomes much more difficult. This combination is recommended only for those parts of the analogue where possible inaccuracies exercise hardly any influence on the final result. Fresh-salt water problems can be solved by the approximation-method of Dagan.

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